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AIR FORCE RUNOFF MODEL (AFRUM) USER MANUAL DOCUMENTATION

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Air Force Runoff Model

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20. ABSTRACT (Continue on reverse side if necessary and identify by block number)
The Air Force Runoff Model (AFRUM) is a stormwater runoff simulation model designed to predict stormwater flow and quality resulting from real or design storms for small watersheds generally limited to 2,000 acres or less. The principal model inputs are watershed area, land use characteristics, percent forested, percent impervious, and percent denuded. The input will also include either an observed hydrograph or an estimated Soil Conservation Service Curve Number (CN). The model is based upon 410 storms in 36 watersheds. Output is both tabular and graphical and provides the watershed hydrograph, pollutograph,

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INCLASSIFIED SECURITY CLASSIFICATION OF THIS PAGE(When Date Entered) 20. ABSTRACT (Concluded). > and discussion of model assumptions. This report provides a background of the model, batch user instructions for CDC 6600 computers, and two example problems. The complete software package is provided in the Appendix.		

PREFACE

This report contains the Air Force Runoff Model (AFRUM) computer program user instructions and documentation developed during the period October 1978 - July 1980 under contract F08635-77-C-0254 with the University of Tennessee, Knoxville, Tennessee 37916. Captain George W. Schlossnagle managed the project and modified the final product with the assistance of Sgt Mike Siebert to improve the user orientation.

A special thanks is given to the Grissom AFB civil engineers, civil engineering personnel and weather personnel, and the 6th Weather Squadron personnel of Tinker AFB for their excellent support of the project's data collection phase.

This report has been reviewed by the Public Affairs Office (PA) and is releasable to the National Technical Information Service (NTIS). At NTIS it will be available to the general public, including foreign nationals.

This technical report has been reviewed and is approved for publication.

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SECTION I

INTRODUCTION

The Air Force Runoff Model (AFRUM) for simulating storm hydrographs and pollutant yields was developed for the U.S. Air Force by the Department of Civil Engineering at the University of Tennessee, Knoxville. The results of three separate but complementary studies had the same fundamental objectives, i.e., to evaluate the effects of specialized land use on stormwater runoff and its associated quality. The three studies were funded by the following three Federal agencies and dealt with the land use indicated:

		Study	
Federal Agency	Contract-Grant No.	Period	Land Use
U.S. Department of Energy	EY-76-S-05-4946	1975-79	Coal Strip
U.S. Department of Interior Office of Water Resource Technology		1976-78	Urbanization
U.S. Air Force	F08635-77-C-0254	1975-79	Air Force

AFRUM was developed in the course of analyzing 410 storms observed on 36 watersheds. The storm sample included watersheds

in agricultural, urban, and 100 percent forested land use conditions as well as watersheds undergoing coal strip mining and three watersheds at Grissom AFB, Indiana. AFRUM was extensively modified by the Air Force Engineering and Services Center, Tyndall Air Force Base, Florida to increase the user utility by modification of input and output formats and simplification of internal processes.

SECTION II

OBJECTIVE OF AFRUM

AFRUM was developed for the purpose of simulating stormwater hydrographs from a realtime or design storm rainfall distribution, land use and soil type of the watershed of interest. The watershed is considered to be a lumped system, and the required basin characteristics are percent of watershed that is forested (PF), percent that is impervious (PI), percent in strip mining or denuded (PS), and surface drainage area in square miles (SQMI).

Runoff volume and the associated rainfall excess time distribution are simulated from the input rainfall using the U.S. Soil Conservation Service Curve Number Model (Reference 1). If a runoff hydrograph (stormwater flow rates out of the basin) is read into the program associated with a rainfall hyetograph (rainfall distribution), the program will compute a Curve Number (CN) from this information. Otherwise, a CN must be read into the computer and would be simulated using the procedures specified in Reference 1.

AFRUM makes provision for simulating a unit hydrograph or unit response function (URF), which is convoluted with the rainfall excess hyetograph. This computer program is an adaptation of the simulation phase of the TVA double triangle model reported by Ardis (Reference 2) and later modified by Betson (Reference 3).

A more detailed description of the development of AFRUM has been reported by Overton, Troxler and Crosby (Reference 4), Overton and Crosby (Reference 5), and CEEDO-TR-77-18.

AFRUM also simulates pollutant loads for the associated storm. This simulation is a function of the specified watershed and storm characteristics. Hence, additional parameters are not needed.

SECTION III

HARDWARE AND SOFTWARE REQUIREMENTS

The computer model described in this manual is written in Fortran IV and operates on CDC 6600 computers under the NOSBE operating system. The program requires approximately 56K bytes of usable core capacity and approximately 6CPU seconds to compile and execute. Input is accomplished by a card reader, and the output is accomplished by a 132 position line printer. The plot subroutine, HYPLOT, within this program utilizes a Calcomp Electromechanical Plotter.

1. Input Requirements

AFRUM accepts the following input:

- a. Accumulated storm rainfall at equal time intervals,
- b. Stormwater discharge flow rates at equal time intervals, DT (this hydrograph may be used if available).
 - c. Watershed characteristics
- (1) Curve Number (CN) which can be estimated using Table 1. If the hydrograph is read in, CN is calculated.
 - (2) Drainage area in square miles (SQMI).
 - (3) Percent forest (PF).

TABLE 1. RUNOFF CURVE NUMBERS FOR URBAN AND SUBURBAN AREAS

(ANTECEDENT MOISTURE CONDITIONS II AND III; I = 0.2S)**

	Curve	numl	oers l	by ant	ecede	nt moi	sture	conc	iltions
]	II			:	III	
	Percent	Hy	irolog			Hyc		gic Sc	
Zoning Classification	Imperviousness	A	B	C	D	A	B 	C	D
Business, industrial, or commercial	75	82	88	90	91	92	95	96	97
Apartment houses	65	78	85	88	90	90	94	95	96
Schools	45	68	78	84	87	84	90	93	95
Urban residential (Lots ± 10,000 ft ²)	40	65	77	83	86	82	89	93	94
Suburban residential (Lots <u>+</u> 12,000 ft ²)	35	62	76	82	85	80	89	92	94
Suburban residential (Lots <u>+</u> 17,000 ft ²)	30	60	74	81	84	78	88	92	93
Suburban residential	25	58	72	80	84	77	86	91	93
Parks and cemetaries	20	55	71	79	83	74	86	91	93
Unimproved areas	15	53	70	78	92	73	85	90	92
Lawns	0	45	65	75	80	66	82	88	91
Woods	0	36	60	73	79	-	-	-	-
Meadow (permanent)	0	30	58	71	78	-	-	-	_
Pasture or range	0	49	69	79	84	-	-	-	-

SCS National Engineering Handbook, Section 4, Table 15.1 "Percent of Imperviousness for Various Densities of Urban Occupancy."

^{*}Soils are divided into four hydrologic soils groups: A, B, C, and D. Group A soils have a high infiltration rate even when thoroughly wet. When thoroughly wet, group B soils have a moderate infiltration rate, group C soils have a slow infiltration rate, and group D soils have a very slow infiltration rate. Table 7.1 of Reference 1 lists more than 9,000 soils and their hydrologic group.

^{**}Antecedent Moisture Condition II is an average condition where III has the highest runoff potential. Condition II means soils in the watershed are practically saturated from antecedent rains.

⁻ Data unavailable.

- (4) Percent impervious (PI).
- (5) Percent strip mined or denuded (PS).

2. Output provided

- a. Title banner page.
- b. Introduction to the model.
- c. Discussion of assumptions made by user.
- d. User input data.
- e. Adjusted model parameters.
- f. Simulated storm hydrograph (graphical and tabular).
- g. If read in, observed storm hydrograph (graphical and tabular).
- h. Suspended sediment storm load, Mn, Fe, Ca, Mg, $\text{SO}_{\slash},$ and total alkalinity storm loads.

SECTION IV

DESCRIPTION OF AFRUM

1. Simulation of Direct Storm Runoff.

This program simulates direct runoff volume and rates using the Curve Number model of the U.S. Soil Conservation

Service (Reference 1). All losses except evapotranspiration are lumped into a single initial abstraction. The model correlates the rainfall-direct runoff relations as a function of soil type, land use, and hydrologic condition.

For simple storms (high intensity and of short duration), the retention relative to the potential maximum retention S bears the following relation:

$$\frac{P - Q - I_a}{S} = \frac{Q}{P - I_a} \tag{1}$$

where P is storm rainfall in inches, Q is the direct storm runoff or effective rainfall in inches, and I_a is the initial abstraction in inches which is a measure of antecedent moisture. Initial abstraction includes surface storage, interception, and infiltration prior to runoff. The concept behind this method is for a given basin soil and land use condition, there is a maximum possible retention as storm rainfall increases, storm runoff will increase as defined by Equation (1). Storm runoff can be solved as follows:

$$Q = \frac{(P - I_a)^2}{(P - I_a) + S}$$
 (2)

An empirical relation (Reference 6) for initial abstraction ($I_a = 0.2S$) is inserted and Equation (2) becomes

$$Q = \frac{(P - 0.2S)^2}{P + 0.8S}$$
 (3)

which is the relation used in the SCS method of estimating direct runoff from storm rainfall.

The runoff equation has been placed in graphical form with the main parameter being watershed retention, S, where S was related to the Curve Number, CN, as

$$S = \frac{1000}{CN} - 10 \tag{4}$$

Equations (2) and (4) show the interrelationships between CN, soils, land use, and hydrologic condition for average antecedent moisture conditions and are shown in the SCS Engineering Handbook (Reference 1) and summarized in Table 1.

Although Equation (3) was intended to simulate runoff volume it has been used extensively to simulate runoff rates by allowing P and Q in Equation (3) to represent accumulated storm rainfall and runoff volumes. Hence, incremental rainfall excess rates in each time period $\mathbf{i}_{\mathbf{p}}(\mathbf{j})$ are simulated as

$$i_e(j) = \frac{Q(j) - Q(j-1)}{DT}$$

where Q(j) and Q(J-1) are accumulated storm runoff volume at times j*DT and (j-1)*DT, respectively, where j is the time increment number.

- 2. Simulation of Storm Hydrograph
 - a. Normalized Unit Response Function (NURF)

The Unit Response Function (URF) in AFRUM is based upon Ardis' (Reference 2) quadrilateral function. The URF was coupled with the CN model to form the TVA double triangle model. The shape of the URFs and associated CNs have been optimized on a total of 410 storms in 36 watersheds.

The quadrilateral URF is based on the concept that the initial response from a watershed comes from the riparian areas, and as other areas of the watershed become saturated, they too begin to contribute to runoff in the form of a delayed response. It is assumed that these two responses can be simulated by two separate triangle response functions as shown in Figure 1. When added together, these two triangles form the quadrilateral unit response function.

Symbols used in the figure are:

I = Precipitation excess intensity in inches per hour. Since the volume of input is one basin-inch = 1/DT.

DT = Time interval used in abstracting rainfall and discharge record in hours.

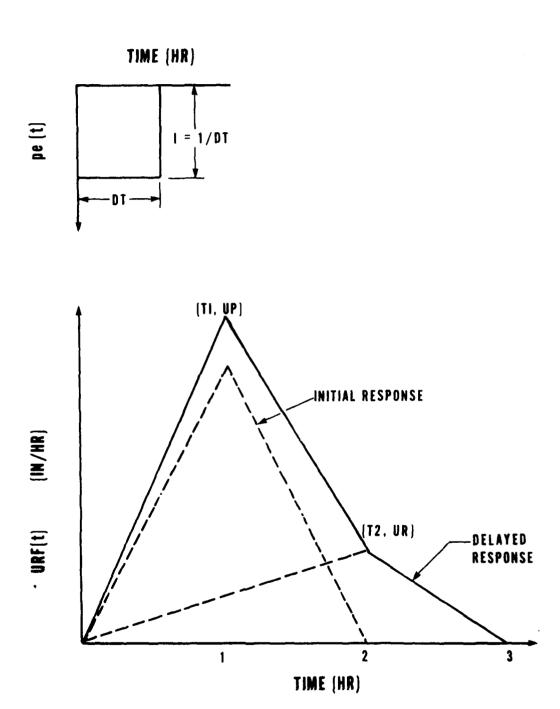


Figure 1. Double Triangle Model for Unit Response Function.

UP = Peak of unit response function at T1.

UR = Peak of delayed response at T2.

T1 = Time to peak of initial response, hours.

T2 = Time base of initial response and equal to the time of peak of delayed response, hours.

T3 = Time of end of delayed response, hours.

pe(t) = precipitation excess as a function of time, t, in inches per hour.

URF(t) = unit response function ordinate as a function of time, t, in inches per hour.

In deriving the URF, it is assumed that the peak of the delayed response (UR) occurs at the end of the initial response (T2), and the time bases of both responses and the time to peak of the initial response are integer multiples of DT. No assumption is made concerning the relative volumes contained in the initial and delayed responses or concerning the relative magnitudes of the peaks of the individual responses.

The double triangle URF is defined by the five parameters UP, UR, T1, T2, and T3. T3 is determined by:

T3 = (NOBS - NRAIN + 1) * DT

where NOBS = number of storm hydrograph ordinates in multiples of DT and NRAIN = number of rainfall increments in multiples of DT.

By maintaining a unit volume, UR is calculated from:

$$UR = (2 - [UP*T2]/[T3-T1])$$

Therefore, defining a storm URF involves determining values of UP, T1, and T2.

The parameters UP, T1, and T2 are optimized using the pattern search technique. The objective function is the minimization of the sum of squares of errors between observed and simulated discharges. Since all five parameters describing the model are allowed to vary from storm to storm, the model is considered nonlinear. Rainfall excess is optimized using the SCS-Curve Number model after setting it equal to the observed direct runoff volume.

The variability of the URF from storm to storm within a watershed was explained by normalizing the time and discharge scale by the associated URF lag time, TL, where TL = time lapse between occurrences of 50 percent of the rainfall excess block and 50 percent of the URF volume. These normalized URFs are referred to as NURFs.

The NURF for each major land use category has been identified empirically, and are shown in Figure 2. The categories are: strip mined, 100 percent forest, urban without extensive storm sewers, urban with extensive storm sewers, and agricultural. As a matter of providing a reference, the NURF

____ 100% FOREST

- - - URBAN

---- AGRICULTURAL

--- SHEET SURFACE RUNOFF

--- URBAN WITH EXTENSIVE STORM SEWERS

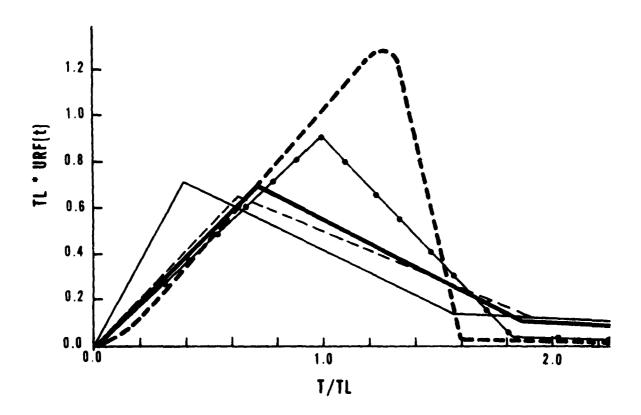


Figure 2. NURF for Various Land Use Conditions.

observed for sheet surface runoff from a plane (Reference 6) is shown. The strip mined watersheds were previously in 100 percent forested areas. Hence, a pattern is shown where the forested and strip mined watersheds have a small initial response whereas the urban and agricultural watersheds have a larger initial response. This implies that imperviousness and crop land produce much more surface runoff. The storm sewered NURF has an even higher initial response, which is generated by runoff collection systems. Sheet surface runoff has no delayed response.

b. Lag Time

Lag time, TL, for a storm is simulated in AFRUM using the concept that it varies inversely with the rainfall excess intensity. AFRUM uses a weighted rainfall excess intensity (WRE)

WRE =
$$\sum_{j=1}^{N} i_e^2(j) / \sum_{j=1}^{N} i_e(j)$$
 (8)

where N is the number of time intervals equal to DT. The weighted storm rainfall intensity is given the exponent 0.4 (References 4 and 5). The following equation is used to calculate the time lag

TL (min) =
$$\mu/WRE^{0.4}$$
 (9)

where lag modulus, μ , is empirically related to watershed characteristics in the following manner:

(1) For rural watersheds

$$\mu(hr) = 0.060 * SQMI + 0.0203 *PF + 1.16$$
 (10)

(2) For urban watersheds

$$\mu(hr) = 3.24 (SQMI/PI)^{0.6}$$
 (11)

c. Convolution of URF

The final step in simulating a stormwater hydrograph is to convolute the storm URF with the rainfall excess time distribution. The URF is defined by simulating storm lag time from Equation (9) using with either Equation (10) or (11), depending on the watershed land use. WRE is calculated by Equation (8).

3. Simulation of Pollutant Yield

a. Load Modulus

AFRUM simulates storm pollutant yield using a load modulus (lb/acre-in of storm runoff) as a function of percent stripped or denuded (PS), lag modulus (μ), and percent forest or trees (PF) in the following form:

$$\mu_{W} = C_{1} * PS - C_{2} * \mu * PF + C_{3}$$
 (12)

The coefficients were optimized using stormwater quality data on a total of eleven watersheds; six of them were undergoing coal

strip mining and five were urbanized (Reference 5). Equation (12) was derived from a mass balance, and each of its terms represents a component of pollutant yield.

C₁ * PS = source of pollutant or soil loss

 $C_2 * \mu * PF = deposition between source and outfall, and$

 C_3 = storage in watershed picked up and redeposited.

The coefficients optimized for the coal strip mined and urbanized watersheds are shown in Table 2. The coefficients were previously presented in References 4 and 5.

b. Storm Load

Once load modulus for the watershed pollutant has been simulated, the storm pollutant yield, SPY, is simulated by

$$SPY = \mu_W * AREA * 640 * SRO$$
 (13)

where SRO is the total storm runoff in surface inches, and AREA is in acres.

TABLE 2. COEFFICIENTS IN POLLUTANT YIELD MODEL
FOR EQUATION (12)

URBAN

		Coefficients				
Pollutant	Source ^C 1	Deposition ^C 2	Storage ^C 3			
Suspended Solids	16.7	21.5	62.3			
Рe	0.442	0.568	1.54			
Mn	0.0072	0.0092	0.20			
Ca	0.147	0.189	2.45			
Mg	0.0597	0.113	0.0323			
Sulfate	0.0719	0.0216	1.24			
Total Alkalinity	0.319	0.128	6.71			
COAI	L STRIP MINED (Ac	tive)				
Suspended Solids	18.2	1.28	576.7			
Fe	0.323	0	0.12			
Mn	0.0161	0	0.002			
Ca	0.131	0	0.38			
Mg	0.133	0	0.40			
Sulfate	1.39	o	2.45			
Total Alkalinity	0.19	0	2.00			

SECTION V

INSTRUCTIONS FOR DATA PREPARATION

The data cards should be prepared according to Figure 3 and Table 3. Figure 3 shows the layout of the data cards in the order in which they must be read into the computer. Table 3 shows how the data are to be punched and lists the description of variables used in the program.

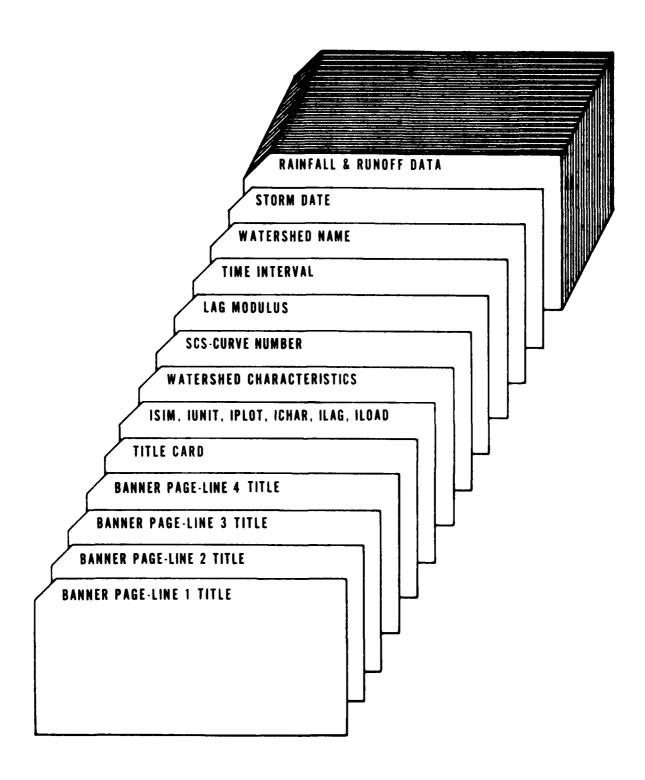


Figure 3. Data Deck for AFRUM

TABLE 3. AFRUM CARD DATA

U.S. AIR FORCE RUNOFF MODEL (AFRUM)

A STORM HYDROGRAPH SIMULATION MODEL DEVELOPED FOR THE AIR FORCE ENGINEERING SERVICES CENTER, TYNDALL AFB, FLORIDA BY DR DONALD OVERTON OF THE UNIVERSITY OF TENNESSEE, PRINCIPAL INVESTIGATOR AND GEORGE W. SCHLOSSNAGLE, CAPT USAF, BSC, PROJECT OFFICER

AFRUM

		AP ROM
CARD	FORMAT/	
NO.	COLUMNS	VARIABLE NAMES AND DESCRIPTION BANNER PAGE - 1ST LINE USER DESCRIPTION
1	(12A1)/	BANNER PAGE - 1ST LINE USER DESCRIPTION
_	(1-12)	201 MIN 201 MIN DECONAL 22011
2	(12A1)/	BANNER PAGE - 2ND LINE USER DESCRIPTION
ے		DANNER FAGE - ZND LINE USER DESCRIPTION
_	(1-12)	
3	(12A1)/	BANNER PAGE - 3RD LINE USER DESCRIPTION
	(1-12)	
4	(12A1)/	BANNER PAGE - 4TH LINE USER DESCRIPTION
	(1-12)	
	· /	
5	(20A4)/	TITLE
,	(20A4)/ (1 80)	
6	(1-80)	NAME AND LOCATION OF WATERSHED
O	(614)/	ISIM, IUNIT, IPLOT, ICHAR, ILAG, ILOAD
	(1-4)	ISIM-A CONTROL VARIABLE TO SPECIFY THE TYPE OF
		DATA SUPPLIED TO THE PROGRAM
		ISIM=1 ONLY RAINFALL DATA IS SUPPLIED AND AN
		ESTIMATE MUST BE MADE OF THE SCS CURVE
		NUMBER
	(5-8)	IUNIT-A CONTROL PARAMETER TO SPECIFY OUTPUT
	(5-0)	
		UNITS OF SIMULATED HYDROGRAPH
		IUNIT=O UNITS ARE IN INCHES/HR
		IUNIT=1 UNITS ARE IN CFS
	(9-12)	IPLOT-A CONTROL PARAMETER TO DETERMINE IF
	,	OUTPUT IS PLOTTED
		IPLOT=0 NO PLOT IS PRODUCED
		IPLOT=1 A PLOT IS PRODUCED
	12 16)	
(13-16)	ICHAR-A CONTROL PARAMETER TO SPECIFY IF LAND
		USE IS URBAN (WITH OR WITHOUT EXTENSIVE
		STORM SEWERS), AGRICULTURAL, COAL STRIP
		MINED OR 100% FOREST
		ICHAR=O URBAN WITHOUT STORM SEWERS
		ICHAR=1 URBAN WITH STORM SEWERS
		ICHAR=2 COAL STRIP MINED
		ICHAR=3 AGRICULTURAL
		ICHAR=4 FORESTED

TABLE 3. AFRUM CARD DATA (CONTINUED)

```
(17-20)
                  ILAG-A CONTROL PARAMETER TO DETERMINE IF LAG
                       MODULUS IS READ IN OR SIMULATED FROM
                       WATERSHED CHARACTERISTICS
                  ILAG=0 LAG MODULUS IS READ IN
                          LAG MODULUS IS SIMULATED FROM URBAN
                  ILAG=1
                          WATERSHED CHARACTERISTICS (% IMPERVIOUS
                           AND AREA [SQMI])
                          LAG MODULUS IS SIMULATED FROM RURAL
                  ILAG=2
                          WATERSHED CHARACTERISTICS (% FOREST AND
                           AREA [SQMI])
    (21-24)
                  ILOAD-A CONTROL PARAMETER TO DETERMINE HOW
                         SUSPENDED SEDIMENT STORM LOAD IS TO BE
                         SIMULATED FROM % FOREST, % IMPERVIOUS,
                         % STRIPPED AND LAG MODULUS
                  ILOAD=1
                            URBAN WATERSHED
                  ILOAD=2 COAL STRIP MINED WATERSHED
WATERSHED CHARACTERISTICS: ID URBAN, PS=0; IF RURAL, PI=0
      (4F8.0)/
                  SQMI, PF, PS, PI
      (1-8)
                  SQMI=WATERSHED AREA IN SQUARE MILES
      (9-16)
                  PF=PERCENT FOREST
      (17-24)
                  PS=PERCENT COAL STRIP MINED (OR DENUDED)
CARD A IS NOT REQUIRED IF ISIM=0 (CARD 5)
      (F10.0)/
                  CN
      (1-10)
                  CN=SCS CURVE NUMBER FOR WATERSHED AT THE TIME
                     THE STORM OCCURED (VARIES WITH ANTECEDENT
                     MOISTURE)
CARD 9 IS NOT REQUIRED IF ILAG=1 or 2 (CARD 5)
      (F10.0)/
      (1-10)
                  U=LAG MODULUS, HOURS
10
      (F10.0)/
      (1-10)1
                  DT=TIME INTERVAL USED IN ABSTRACTING RAINFALL
                  (HRS)
CARD 11,12, and 13 MUST BE REPEATED FOR EACH STORM TO BE
SIMULATED
11
      (8A4)/
                  BASIN
                  BASIN=NAME OF WATERSHED
      (1-32)
```

TABLE 3. AFRUM CARD DATA (CONCLUDED)

12	(2A4)/	BDATE BDATE=DATE OF STORM
13	(I1,9X, F10.3,30X, F10.3)/	<pre>ISTAGE,RAIN(I),SFLOW(I)</pre>
	(1)	ISTAGE-A CONTROL PARAMETER TO SIGNAL THE END OF A STORM DATA SET
		ISTAGE=O SIGNALS CONTINUATION OF RAINFALL/ RUNOFF
	(2.10)	ISTAGE=1 SIGNALS LAST DATA CARD OF A STORM
	(2-10) (11-20)	RAIN(I) RAIN(I)=CUMULATIVE STORM RAINFALL ABSTRACTED AT DT TIME INTERVALS. THE FIRST VALUE OF RAIN(I) MUST BE 0.0 AT TIME 0.0
	(21-50) (51-60)	BLANK SFLOW(I) SFLOW(I)=STORMWATER DISCHARGED IN CFS, ABSTRACTED AT DT TIME INTERVALS, VALUES OF SFLOW(I) ARE NOT REQUIRED IF ISIM=1 (CARD 1). THE FIRST AND LAST VALUES OF SFLOW(I) MUST BE 0.0

NO. OF RAINFALL/RUNOFF CARDS MUST BE EQUAL TO NUMBER OF RAINFALL/RUNOFF OBSERVATIONS A MAXIMUM OF 500 RAINFALL/RUNOFF OBSERVATIONS MAY BE SUBMITTED

SECTION VI

EXAMPLE PROBLEMS

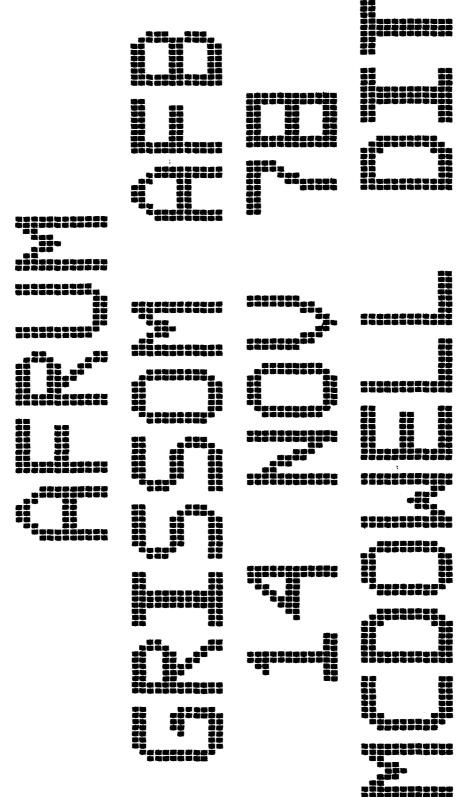
1. McDowell Ditch

An example application of AFRUM is provided for the McDowell Ditch watershed at Grissom Air Force Base, Indiana. The watershed is 276 acres in size, has 20 percent imperviousness, and essentially zero percent trees. The soils are classified in SCS group C and watershed land use is considered to be urban without extensive storm sewers.

The accumulated rainfall and hydrograph for the storm of November 14, 1978 was measured for the McDowell watershed. Since the runoff was available, the storm CN was calculated within AFRUM. However, using the SCS-CN method (see Table 3) the CN for the storm was estimated to be 83, whereas the calculated CN using storm rainfall and runoff was 81. This produced an error of 3.1 percent.

Hence, the data was punched onto the cards into the specified format and entered into the computer after the appropriate JCLs. A listing of the input and the program output follows.

```
AFRUM
GRISSOM AFB
14 NOV 78
MCDOWELL DIT
GRISSOM AFB
0 1 1 0 1
 GRISSOM
1 1 0
0.43 5.5
0.25
MCDOWELL DITCH
11-14-78
                                                   0.0
                                                                    22.0
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THE U.S. AIR FORCE RUNDEF MODEL (AFRUM) FOR SIMULATING STORM
HYDROGRAPHS AND POLLUTANT YIELDS WAS DEVELOPED FOR THE AIR FORCE
BY THE DEPARTMENT OF CIVIL ENGINEERING. UNIVERSITY OF TENNESSEE.
KNOXVILLE AND THE RESEARCH AND DEVELOPMENT DIRECTORATE OF THE
BASE FORCE ENGINEERING SERVICES CENTER (AFESC). TYNDALL AIR FORCE
BASE THE D.S. AIR FORCE WAS THE PROJECT OF THE RESULTS OF THE CASE AS THE DRIVESTIGATOR AND DR. GEORGE W. SCHLOSSNAGLE
SEPARATE BUT COMPLIMENTARY STUDIES WERE INCORPORATED INTO AFRUM.
SEPARATE BUT COMPLIMENTARY STUDIES WERE INCORPORATED INTO AFRUM.
THE COMPLIMENTARY STUDIES WERE FUNDED BY THE U.S. DEPART—
THE STUDY PERIOD WAS FROM 1973 THROUGH 1979. AND THE U.S. AIR FORCE.
THE STUDY PERIOD WAS FROM 1973 THROUGH 1979. AFRUM STORM EVENTS ON 36 WATERSHEDS.
WHICH INCLUDED AGRICULTURAL, URBAN AND FORESTED LAND USE CONDITIIONS
BASE.

THE N AFRUM WAS DEVELOPED FOR THE PURPOSE OF SIMULATING STORMWATER HYDROGRAPHS FROM ACTUAL OR DESIGN STORM RAINFALL DISTRIBUTIONS.
LAND USE CHARACTERISTICS AND SOIL TYPE IN THE WATERSHED OF CONCERN.
LAND USE CHARACTERISTICS AND SOIL TYPE IN THE WATERSHED OF CONCERN.

LAND USE CHARACTERISTICS AND SOIL TYPE IN THE WATERSHED OF CONCERN.

LAND USE CHARACTERISTICS AND SOIL THAT IS IMPERVIOUS

S. PERCENT OF WATERSHED THAT IS IMPERVIOUS

ANTERON TOF WATERSHED THAT IS DENUDED OR STRIP MINED

ARE SIMULATED FROM INPUT ASSOCIATED RAINFALL EXCESS TIME DISTRIBUTION

ARE SIMULATED FROM INPUT ARUNOFF HYDROGRAPH IS PROVIDED WITH THE ASSOCIATED RAINFALL USING HYDROGRAPH IS PROVIDED WITH THE ASSOCIATED RAINFALL HYETOGRAPH, AFRUM WILL COMPUTE THE CURVE NUMBER.

BER, CN; OTHERWISE CN MUST BE AN INPUT. CN IS SELECTED BY USING THE PROCEDURES PROVIDED IN THE NATIONAL ENGINEERING HANDBOOK.

R SIMULATING A WHICH IS CONVO ALSO SIMULATES ATION IS A FU AFRUM MAKES PROVISION FOR UNIT RESPONSE FUNCTION. WH CESS HYETOGRAPH. AFRUM ALINDUTED STORM. THIS SIMUL/WATERSHED AND STORM CHARAC

GRISSOM AFB

THE WATERSHED NAME IS MCDOWELL DITCH
THE DATE OF THE RAINFALL EVENT IS 11-14-78
THE WATERSHED SIZE IN SQUARE MILES IS: SQMI=
THE PERCENT OF THE BASIN WHICH IS FOREST IS: PF=
THE PERCENT OF THE BASIN DENUDED IS; PS=
THE PERCENT OF THE BASIN WHICH IS IMPERVIOUS IS: PI=
22.00

BY SETTING ISIM = 0 YOU HAVE INDICATED THAT YOU PLAN TO SUPPLY EITHER ACTUAL OR DESIGN RAINFALL AND RUNOFF DATA; THEREFORE, THE SCS CURVE NUMBER, CN, WILL BE DETERMINED BY SETTING RUNOFF EQUAL TO PRECIPITATION EXCESS.

OUTPUT UNITS OF THE SIMULATED HYDROGRAPH ARE HOURS AND CUBIC FEET PER SECOND, CFS. AS SELECTED BY SETTING IUNIT = 1.

YOU HAVE REQUESTED THAT THE HYDROGRAPH BE PLOTTED BY SETTING IPLOT = 1.

THE WATERSHED IS ASSUMED TO BE URBAN WITHOUT STORM SEWERS; ICHAR = 0.

YOU HAVE ELECTED TO SIMULATE LAG MODULUS IN HOURS ASSUMING IT IS A FUNCTION OF THE WATERSHED AREA AND PERCENT IMPERVIOUS AREA OF THE WATERSHED; ILAG = 1.

SEDIMENT LOAD HAS BEEN SIMULATED ASSUMING THE WATERSHED IS RURAL OR URBAN WITHOUT MAJOR CONSTRUCTION; ILOAD = 1.

TABLE I. USER INPUT

TIME (HR)	TOTAL RAIN (IN)	INCREMENTAL RAIN (IN)	OBSERV FLOW (CFS)	(ED DATA FLOW (IN/HR)
05050505050505050505050505050505050505	05050000001450385000N00841346678900368856777889000000000000000000000000000000000	0555500000131535750008867&1211103332071110010100000000000000000000	00000000000000000000000000000000000000	00000000000000000000000000000000000000

TABLE I. USER IMPUT (CONTINUED)

05050505050505050505050505050505050505	00000000000000000000000000000000000000		74000000000000000000000000000000000000	4648913555557599711582604888888064400668999999995439585117466663339 6851766444446444655844740655862888888753222119666665543110000000000000000000000000000000000
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TABLE I. USER INPUT (CONCLUDED)

AFI .085	90E•	24. 0	TL .820	UP .809	UR .146	71 .500	1.500	5.750	AREA 991
ADJ	ADJUSTED HYDROGRAPH	OGRAPH PAR	AMETERS						
UP • A16	UR • 148	T1 .500	12 1.500	T3 5.750	AREA 1.000				

TABLE II. HYDROGRAPH OUTPUT

OHSERVED FLOW OUT OF WATERSHED (CFS)	PREDICTED FLOW OUT OF WATERSHED (CFS)	TIME (HOURS)	SUSPENDED SOLIDS DISCHARGED FROM WATERSHED LBS/SEC
00000000000000000000000000000000000000	00000000000000004624798002655259484248050856060528432347078 000000000000000046247986493974872036372895358240815063115 000000000000000433674473649397487203637289538240815063115 0000000000000000004336746901135711809588121212520755544322211111	00000000000000000000000000000000000000	00000000000000000000000000000000000000

TABLE II. HYDROGRAPH OUTPUT (CONTINUED)

00000000000000000000000000000000000000	3881605300000000000000000000000000000000000	00000000000000000000000000000000000000	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\
470 470 430 430 3320 3320 3320 3222 3222 3222	0.000 0.000	27000000000000000000000000000000000000	00000000000000000000000000000000000000

TABLE II. HYDROGRAPH OUTPUT (CONCLUED)

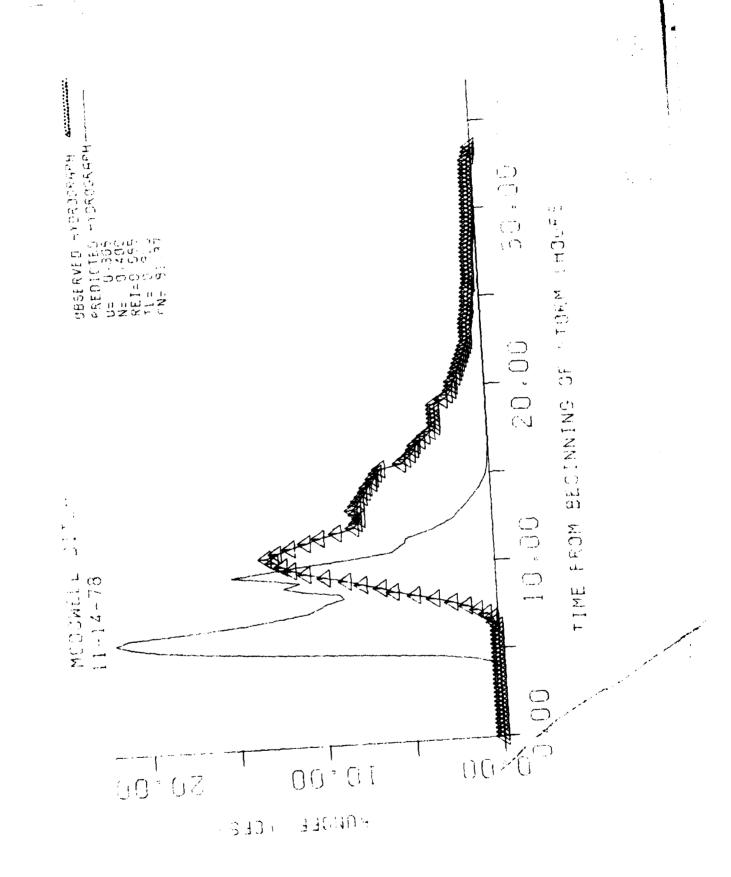
.240	0.000	31.500	0.000
.240	0.000	31.750	0.000
.230	0.000	32.000	0.000
•230	0.000	32.250	0.000
• 220	0.000	32.500	0.000
.220 .100	0.000 0.000	32.750 33.000	0.000
.050	0.000	33.250	0.000
0.000	ŏ.ŏŏŏ	33.500	0.000

TOTAL SUSPENDED SOLIDS (LBS) DISCHARGED FROM WATERSHED DURING STORM EVENT = 2308.209

FE CONC (PPM) FE LOAD (LBS)	6.80680 135.87454	
MN CONC (PPM) MN LOAD (LBS)	.88400 17.64604	
CA CONC (PPM) CA LOAD (LBS)	10.82900 216.16404	
MG CONC (PPM) MG LOAD (LBS)	•14277 2•84984	
SO4 CONC (PPM) SO4 LOAD (LBS)	5.48080 109.40547	
ALK CONC (PPM) ALK LOAD (LBS)	29.65820 592.02478	

.663	KT1	KT2	KUR
	.632	1.880	•120
.306	N • 400	DT •25000	

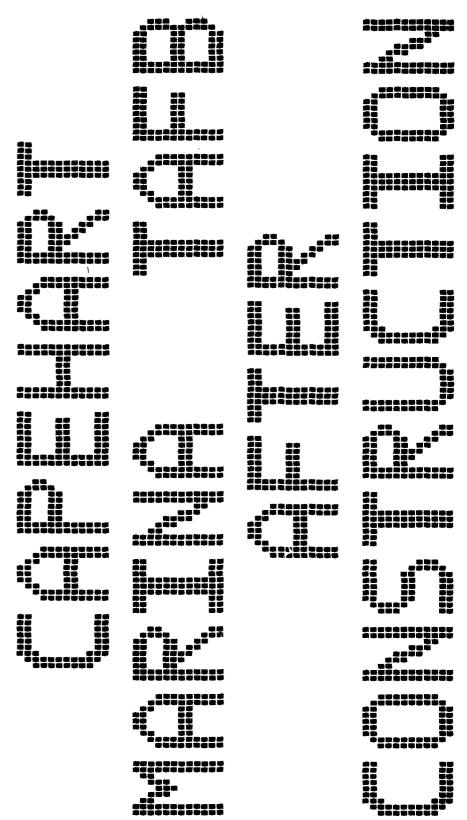
SIMULATED LOAD MODULUS(LBS/AC-IN OF RUNOFF) 26.16



2. Capehart Marina

An example is provided for the Capehart Marina watershed at Tyndall AFB, Florida. No runoff data is available, and a design storm is supplied. The design storm of interest is 10 inches of rainfall in 12 hours read in at increments of one tenth of an hour. The watershed area is 0.17 square mile with 14.5 percent forest, 1 percent denuded area, and 15 percent impervious area. Curve number 58 was selected from Table 1. The watershed is urban with storm sewers.

A listing of the input and output follows.



INTRODUCTION

THE U.S. AIR FORCE RUNOFF MODEL (AFRUM) FOR SIMULATING STORM HYDROGRAPHS AND POLLUTANT YIELDS WAS DEVELOPED FOR THE AIR FORCE BY THE DEPARTMENT OF CIVIL ENGINEERING, UNIVERSITY OF TENNESSEE, KNOXVILLE AND THE RESEARCH AND DEVELOPMENT DIRECTORATE OF THE AIR FORCE ENGINEERING SERVICES CENTER (AFESC), TYNDALL AIR FORCE BASE, FLORIDA. DR. DONALD E. OVERTON OF THE UNIVERSITY OF TENNES—SEE WAS THE PRINCIPAL INVESTIGATOR AND DR. GEORGE W. SCHLOSSNAGLE OF THE U.S. AIR FORCE WAS THE PROJECT OFFICER. THE RESULTS OF THREE SEPARATE BUT COMPLIMENTARY STUDIES WERE INCORPORATED INTO AFRUM. EACH OF THE THREE STUDIES HAD THE FUNDAMENTAL OBJECTIVE TO EVALUATE THE EFFECTS OF SPECIALIZED LAND USE ON STORMWATER RUNOFF AND ITS ASSOCIATED QUALITY. THE THREE STUDIES WERE FUNDED BY THE U.S. DEPARTMENT OF ENERGY, THE U.S. DEPARTMENT OF INTERIOR AND THE U.S. DEPARTMENT OF ENERGY, THE U.S. DEPARTMENT OF INTERIOR AND THE U.S. AIR FORCE. THE STUDY PERIOD WAS FROM 1975 THROUGH 1979. AFRUM WAS DEVEL—OPED IN THE COURSE OF ANALYZING 410 STORM EVENTS ON 36 WATERSHEDS, WHICH INCLUDED AGRICULTURAL, URBAN AND FORESTED LAND USE CONDITIIONS AS WELL AS COAL STRIP MINING AND THREE WATERSHEDS ON A U.S. AIR FORCE BASE. BASE .

AFRUM WAS DEVELOPED FOR THE PURPOSE OF SIMULATING STORMWATER HYDROGRAPHS FROM ACTUAL OR DESIGN STORM RAINFALL DISTRIBUTIONS, LAND USE CHARACTERISTICS AND SOIL TYPE IN THE WATERSHED OF CONCERN. THE REQUIRED INPUT BASIN CHARACTERISTICS ARE:

1. PERCENT OF WATERSHED THAT IS FORESTED
2. PERCENT OF WATERSHED THAT IS IMPERVIOUS
3. PERCENT OF WATERSHED THAT IS DENUDED OR STRIP MINED
4. SURFACE DRAINAGE AREA
RUNOFF VOLUME AND THE ASSOCIATED RAINFALL EXCESS TIME DISTRIBUTION ARE SIMULATED FROM INPUT RAINFALL USING THE U.S. SOIL CONSERVATION SERVICE CURVE NUMBER. IF A RUNOFF HYDROGRAPH IS PROVIDED WITH THE ASSOCIATED RAINFALL HYETOGRAPH, AFRUM WILL COMPUTE THE CURVE NUMBER. CN; OTHERWISE CN MUST BE AN INPUT. CN IS SELECTED BY USING THE PROCEDURES PROVIDED IN THE NATIONAL ENGINEERING HANDBOOK, SECTION 4, U.S. SOIL CONSERVATION SERVICE.

AFRUM MAKES PROVISION FOR SIMULATING A UNIT HYDROGRAPH OR UNIT RESPONSE FUNCTION. WHICH IS CONVOLUTED WITH THE RAINFALL CESS HYETOGRAPH. AFRUM ALSO SIMULATES POLLUTANT LOADS FOR THE INPUTED STORM. THIS SIMULATION IS A FUNCTION OF THE SPECIFIED WATERSHED AND STORM CHARACTERISTICS.

CAPEHART MARINA AFTER CONSTRUCTION

THE WATERSHED NAME IS CAPEHART MARINA - (AFTER)
THE DATE OF THE RAINFALL EVENT IS 04-22-80
THE WATERSHED SIZE IN SQUARE MILES IS: SQMI= .17
THE PERCENT OF THE BASIN WHICH IS FOREST IS: PF= 14.50
THE PERCENT OF THE BASIN DENUDED IS; PS= 1.00
THE PERCENT OF THE BASIN WHICH IS IMPERVIOUS IS: PI= 15.00

BY SETTING ISIM = 1 YOU HAVE INDICATED THAT YOU PLAN TO SUPPLY ACTUAL OR DESIGN RAINFALL DATA; THEREFORE, AN ESTIMATE OF THE SCS CURVE NUMBER, CN, MUST BE PROVIDED.

OUTPUT UNITS OF THE SIMULATED HYDROGRAPH ARE HOURS AND CUBIC FEET PER SECOND, CFS, AS SELECTED BY SETTING IUNIT = 1.

YOU HAVE REQUESTED THAT THE HYDROGRAPH BE PLOTTED BY SETTING IPLOT = 1.

THE WATERSHED IS ASSUMED TO BE URBAN WITH STORM SEWERS: ICHAR = 1.

YOU HAVE ELECTED TO SIMULATE LAG MODULUS IN HOURS ASSUMING IT IS A FUNCTION OF THE WATERSHED AREA AND PERCENT IMPERVIOUS AREA OF THE WATERSHED; ILAG = 1.

SEDIMENT LOAD HAS BEEN SIMULATED ASSUMING THE WATERSHED IS RURAL OR URBAN WITHOUT MAJOR CONSTRUCTION; ILOAD = 1.

TABLE I. USER INPUT

TIME (HR)	TOTAL Rain (IN)	INCREMENTAL RAIN (IN)	OBSERV FLOW (CFS)	VED DATA FLOW (IN/HR)
00000000000000000000000000000000000000	05050505050505050500000000000000000000	05555555555555555550000000000000000000	00000000000000000000000000000000000000	

00000000000000000000000000000000000000	0000000005050500000000000000000000505050	0000000000555550000000000000000000005555	000000000000000000000000000000000000000	
11.40 11.50 11.60	9.85 9.85 9.90 9.95	• 05 • 05 • 05	0.00 0.00 0.00	0.00000 0.00000 0.00000 0.00000

ESTIMATED CN= 58.00

AREA 1.001		
2.900		
.500		
11 • 300		
uR •129		AREA 1.000
UP 3•330		73
75.	METERS	12
24.	OGRAPH PAF	7300
.220	ADJUSTED HYDROGRAPH PAR	UR 129
. 600	PD?	UP 3,327

TABLE II. HYDROGRAPH OUTPUT

OBSERVED FLOW OUT OF WATERSHED (CFS)	PREDICTED FLOW OUT OF WATERSHED (CFS)	TIME (HOURS)	SUSPENDED SOLIDS DISCHARGED FROM WATERSHED LAS/SEC
00000000000000000000000000000000000000	00000000000000000000000000000000000000	00000000000000000000000000000000000000	00000000000000000000000000000000000000

00000000000000000000000000000000000000	05764212759950323268567867353809614678888990380193430696123332812086666666799894971605367869258814002172952116055555555555666666667998911921667777777889999958776665925888888888888888888888888888888888	00000000000000000000000000000000000000	4555666677777888990578888669111100000000000000000000000000000
0.000 0.000 0.000 0.000 0.000 0.000 0.000	811-2-0-0-2171805-2116035-6666666683570795-25814002171805-2116035-666666668357079795-25811-2-0-0-217295-2116035-666666683570795-29-6-1-0-2-2-2-2-2-2-2-2-2-2-2-2-2-2-2-2-2-2	99999999999999999999999999999999999999	NACE OF THE PROPERTY OF THE PR

0.000	4 397	12 400	.012
0.000	4.387 3.950 3.537 3.146 2.777 2.432	12.600 12.700 12.800 12.900 13.100 13.200 13.400 13.500	រំបំរំបំ
0.000	3.537	12.800	.011 .010 .009 .008
0.000	3.146	12.900	. 0 0 9
0.000	ž.††Ť	13,000	.008
0.000	2.432	13.100	.007
0.000	2.109	13.200	.007 .006
0.000	1.809	13.300	-005
0.000	1.531	13.400	.004
0.000	2.109 1.809 1.531 1.277 1.045	13.500	.004 .004 .003
0.000	1.045	1.34000	.003
0.000	•837	13.700 13.800	200. 200.
0.000	•651	13.800	•005
0.000	• 489	13.900	.001
0.000	• 349	14.000	.001
0.000	.651 .651 .489 .349 .2140	14.100	.001
0.000	• 140	14.200	.000
0.000	•010	14.300	•000
0.000	0.023 0.000	14.400	0.00
U U U U	U • U U U	144300	0.000

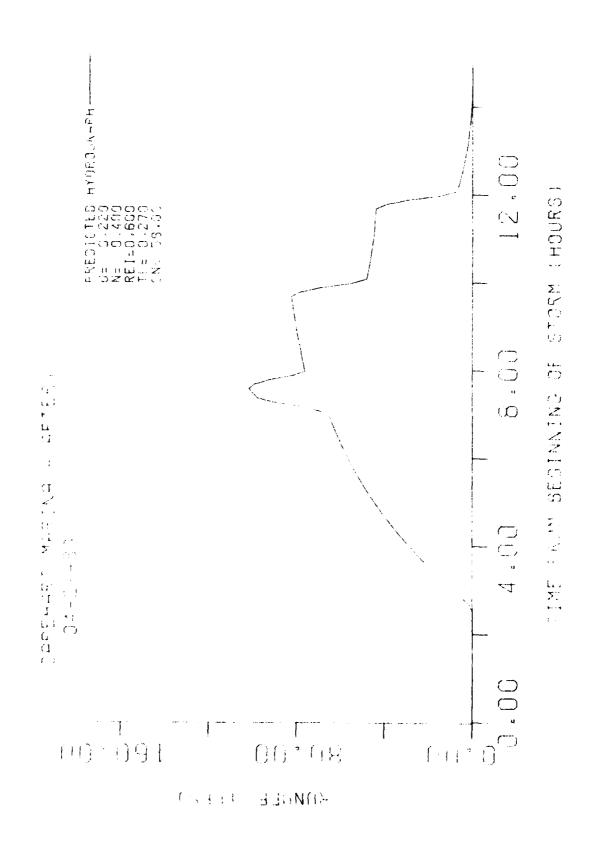
TOTAL SUSPENDED SOLIDS (LBS) DISCHARGED FROM WATERSHED DURING STORM EVENT = 5188.523

FE CONC (PPM)	6.80680
FE LOAD (LBS)	775.87097
MN CONC (PPM)	.88400
MN LOAD (LBS)	100.76246
CA CONC (PPM)	10.82900
CA LOAD (LBS)	1234.34018
MG CONC (PPM)	•14277
MG LOAD (LBS)	16•27314
SO4 CONC (PPM)	5.48080
SO4 LOAD (LBS)	624.72727
ALK CONC (PPM)	29.65820
ALK LOAD (LRS)	3380.58065

OPTIMIZED CURVE NUMBER = 0.0 TOTAL RUNOFF= 4.6306INCHES

KUP KT1 KT2 KUR .900 .956 1.800 .035

SIMULATED LOAD MODULUS(LBS/AC-IN OF RUNOFF) 10.30



(THE REVERSE OF THIS PAGE IS BLANK)

REFERENCES

- 1. U.S. Soil Conservation Service, <u>National Engineering Handbook</u>
 <u>Section 4, Hydrology, Washington, D.C., 1972.</u>
- 2. Ardis, C.V., Jr., "Storm Hydrographs Using a Double Triangle Model," TVA, Division of Water Control Planning, January 1973.
- 3. Betson, R.P., <u>Urban Hydrology A Systems Study in Knoxville</u>, <u>Tennessee</u>, TVA, Division of Water Management, 1976.
- 4. Overton, D.E., W.L. Troxler and E.C. Crosby, "Simulation of Effects of Urbanization of Stormwater Runoff and Quality," Univ. of Tennessee, Water Resource Center, Report No. 74, December 1979.
- 5. Overton, D.E., and E.C. Crosby, "Effects of Contour Coal Strip Mining on Stormwater Runoff and Quality New River Basin, Tennessee," Department of Civil Engineering, The University of Tennessee, Knoxville, 1979.
- 6. Overton, D.E. and M.E. Meadows, <u>Stormwater Modeling</u>, Academic Press Inc., New York, 1976.

APPENDIX A
LISTING OF AFRUM

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10/06/80		d 11 12
FTN 4.8+518	TAPES=INPUT, TAPE6=OUTPUT, TAPE7, TAPE7, TAPER. ***********************************	AND LOCATION OF WATERSHED LUNITS PAGE - ATH LINE USER DESCRIF R PAGE - ATH LINE USER DESCRIF R PAGE - ATH LINE USER DESCRIF R PAGE - ATH LINE USER DESCRIF LUNITS PAGE - ATH LINE USER DESCRIF R PAGE - ATH LINE USER DESCRIF LONITS PAGE ATH LINE USER DESCRIF R PAGE ATH LINE USER DESCRIF R PAGE ATH LINE USER DESCRIF A CONTROL VARIABLE ITA SUPPLIE R PAGE ATH LINE USER DESCRIF A CONTROL PROBLEMENT IS SUPPLIE A CONTROL PROBLEMENT IS DATA IN INC. A CONTROL PARAMETER TO SPECIFY END A CONTROL SPECIFY A CONTRO
4/74 OPT=1	AFRUM (INPUT+OUTP	
•		

9090FFFFFF 9F890-MU481	Ste Entercoのおおちますをごうらむのようになっています。 という Ste Entercoのはようちゃ Entercoのはようらっている But Ste Entercoののいことののいことのなっているないないのいののできない But Ste Entercolory Steel Entercolory Entercolory Entercolory Entercolor Entercolory									
				PAPAPA PRESCUENT PERES						
ILAG- A CONTROL PARAMETER TO DETERMINE IF LAG MODULUS IS READ IN OR SIMULATED FROM LAG MODULUS IS READ IN ILAG=0 LAG MODULUS IS READ IN ILAG=1 LAG MODULUS IS SIMULATED FROM "RBAN AND AREA (SAMI) SIMULATED FROM RURAL ILAG=2 LAG MODULUS IS SIMULATED FROM RURAL ILAG=2 LAG MODULUS IS SIMULATED FROM RURAL INTERPREDICALIS IS INULATED FROM RURAL MATERSHED CAMPACTERISTICS (% FOREST	ILOAD- A CONTROL PARMETER TO DETERMINE HOW SUSPENDED SEDIMENT STORM LOAD IS TO BE SIMULATED FROM % FOREST, % IMPERVIOUS, STRIPPED AND LAG MODULUS ILOAD=I URBAW WATERSHED ATERSHED	SOMI + I PEEE	D IF ISIM=0 (CARD 5) CN= SCS CURVE NUMBER FOR WATERSHED AT THE TIME THE STORM OCCURED (VARIES WITH ANTECEDENT MOISTURE)		DT= TIME INTERVAL USED IN ABSTRACTING RAINFALL(HRS) MUST BE REPEATED FOR EACH STORM TO BE SIMULATED BASIN= NAME OF WATERSHED	DATE OF SIGRM	ISTAGE - A CONTROL PARAMETER TO SIGNAL THE END OF A STORM DATA SET STAGE - SIGNALS CONTINUATION OF RAINFALL/RUNOFF BLANCE: SIGNALS LAST DATA CARD OF A STORM RAINF RAIN(I) - DIMULATIVE STORM RAINFAL ABSTRACTED AT RAIN(I) - DIMULATIVE STORM RAINFALL BESTRACTED AT	TARE IN CE		
17-20		WATERSHED CHARACTERISTIC: (4F8.0) (1-8) (9-16) (17-24) (25-32)	CARD 8 IS NOT REQUIRED 8 (F10,0)	CARD 9 I	CARDS 11.12. AND 13	12 (2A4) 13 (11.90x+10.3)	, -g	(51-50)		
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70 70		\$ 6	95	100	105	115	120	130		

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				TELITIEEEEEEEEEEEEEEEEEEEEEEEEEEEEEEEEE	######################################
OBSERVATIONS OF RAINFALL/RUNOFF CARDS MUST BE EQUAL TO NUMBER OF RAINFALL/RUNOFF OBSERVATIONS A MAXIMUM OF 500 RAINFALL/RUNOFF OBSERVATIONS MAY BE SUBMITTED C. C	>.DUMX(4).DUMY(4).BIGX.OBPEAK,PRPEAK,TPOB.TPPR.ISIM >.IPLOT.UNIT.NPRI.NPRE.AECARUR >.IPLOT.UNIT.NPRI.NPRE.AECARUR >.ILCAD.CO.SUNIT.NPRI.NPRE.AECARUR CILCAD.CO.NING DATA CARDS CONTAIN COFFFICIENTS FOR SIMULATING LAG MODULUS. COP.TIT.TO.AND T23 US A FUNCTION OF MATERSHED CHARACTERISTICS LINEAR CHESE REGIONALIZED COFFICIENTS WERE DERIVED BY USING BMDD4A (LINEAR REGRESSION UDON PRINCIPAL COMPONENTS) CHESE COFFICIENTS ARE SUBJECT TO REVISION AS MORE DATA BECOMES AVAILABLE	^ ^ ^ ^	>8.0.09-0.0156.1.9912.0.0051.0.02849.2.7258.1.18497/ CALL PLOTS(0.0.1.5,-3) PRINTS BANNER (FIRST 4 CRDS) CALL LETTER CALL AESOP	EMAXIMUM DIMENS =800 WATERSHED LOCAT S99) TITLE [[2044] IT (4810)	C READ PROGRAM CONTROL PARAMETERS (CARD 2) READ(5,12) ISIM, IUNIT, IPLOT, ICHAR, ILAG, ILOAD C KEAD WATERSHED PHYSICAL CHARACTERISTICS (CARD 3) C READ WATERSHED PHYSICAL CHARACTERISTICS (CARD 3) I F FORMA (4F8,2) C READ (5,15) GO TO 16 C READ SCS CURVE NUMBER (CARD 4) IS FORMAT (F10,0)
.35 • • • • • • • • • • • • • • • • • • •	991	921	175	85	0 6 00

AAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAA					
CONTINUE RN=.4 WE READ LAG MODULUS (CARD 5) IF (ILAG GT 0) GO TO 20 GO TO 23 U= (0.060*SQM1) + (0.023*PF) + 1.16 IF (U.GE 0.0) GO TO 23 WRITE(0.550 0.0) GO TO 23	F LAGMODULUS SIMULATED, PROGRAM	IF (ICHAR .GT. 0) GO TO 24 KUP=0.663 KTI=0.663 KTI=0.663 KTI=0.663 KTI=0.663 KTI=0.663 KTI=0.663 KTI=0.663 KTI=0.690 IF (ICHAR .GT. 1) GO TO 25 KTI=0.900	X71=0.956 XVR=0.030 XVR=0.030 IF (ICMAR GT. 2) GO TO 26 IF (ICMAR GT. 2) GO TO 26 XVR=0.253 XVR=0.21	IF (ICHAR .6T. 3) 60 TO 27 KUP=6.705 KTI=1.60 KTI=1.81 KUP=6.13 KOP=6.13 KTI=1.60 KUP=6.13 KTI=1.60 KUP=6.13 KTI=1.60 KUP=6.13 KTI=1.60 KTI=1.60 KTI=1.60 KTI=1.60 KTI=1.60 KTI=1.60	KUR=0.10 CONTINUE READ TIME INTERVAL (CARD 6) READ [5.15] DT READ [5.15] DT READ [5.15] DT IF (ILOAD .60.2) DLOAD=18.17+PS-1.28*U*PF+62.3 IF (ILOAD .60.2) 0.00 0.00 0.000 URITO (6.31) WRITE (6.31) FORMAT (1H .///.12X,"WARNING: EQUATIONS PREDICT MORE SEDIMENT CAN
26 CC	22 23 23	ou ∛	52	26	ရိုပ္သပ္ (
205 210 215	220	€ ₹ 58	\$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$	250 255	% % % %

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THE TERMINATION OF THE PROPERTY OF THE PROPERT
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WRITE (6.103)
FORMATINE (5.104)
FORMATINE (5.105)
F
. BE DEPOSITED". / 12x, "THAN CAN BE PICKED UP: THEREFORE. LOAD MODUL. US IS SET FOUAL TO ZERO." . ///)
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                              ## (i) = RAIN(I) - RAIN(I-1)
IF (i) = RAIN(I-1)
IN (II) - LT. 0.0) RF(I) = 0.0
INCTINUE
INCENS OF RUNDFF = ((E_OMICFS)) AREA(FT**2)) *3600SEC/HR*12.0IN/FT
INCENS OF RAIN(I) - (ST. 0.0) NPRF**NPRF*1]
IF (RAIN(I) - GT. 0.0) NPRF**NPRF*1
IF (RAIN(I) - GT. 0.0) NPRF**NPRF*1
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                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                           (CARD 9)
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                          CALL BASINA (SOMI, PF, PS, BDATE, BASIN, PI)
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                           READ OBSERVED RAINFALL AND DISCHARGE
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                   READ(5.60) ISTAGE.RAIN(I).SFLOW(I)
FORMATII,9X.FI0.3330X.FI0.31
TIME(I).2DF4.LDT
IF (RAIN(I).6T, RMAX) RMAX=RAIN(I)
IF (I = GT, I) GO TO 62
RF(I).60.0
GO TO 70
                                                                                                                                                                                                                                                                                                       READ DATE OF STORM (CARD 8)
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                          (Tault .Eq. 1) 60 TO 50
                                                                                                                                                                                                                                                                                                                                                                   READ(5.37) BOATE
FORMAT (48)
TORMAT (15(7),46X*2044,//)
                                                                                                  READ BASIN NAME (CARD 7)
                                                                                                                                                                               READ(5.10) BASIN
IF(EOF(5)) 480.35
CONTINUE
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10/06/80									
FTN 4.8+518	SUBROUTINE SCS(CN) THIS SUBROUTINE COMPUTES AND DISTRIBUTES EXCESS PRECIPITATION	REAL KTI*KT2*KUP*IMP*LOSS*KT23*KUR COMMON RUNOFF(800)*HYDRO(800)*RNOFF(500)*RAIN(500)*STAGE(500) >*AMF(500)*RF(500)*PEI(500)*PE(500)*RAIN(4)*BASIN(4)*BASIE* >*AMFA(664)*AMFA; UPATI)*T2*T3*DF*UR*KEI*KT1*KT2*KDP*RN*ITOTAL* >{CRASH*LDMAX*NPRF*UHYDRO(600)*NPRO*CNOFT*NBIG*IL*KT23*	FLOWI'500),SFLOW'500),BSFLOW(500) BPEAK,PREAK,TPOB,TPPR,ISIM 2-AREA,KUR C.DD.SINU.CNA,SS.IMP.TLP		(I)=RF(I)-PHI I)=RF(I)-PHI GO TO 70 *2-0)/(ARF(I)+0.8*S)		(I)=ARF(I=1)+RF(I)-PHI (O) TO 110	F(I)=0,2*\$)**2.0)/(ARF(I)+0.8*\$)  -SRO(I=1) 	
74/74 OPT=1	SUBROUTINE SCS(CN) THIS SUBROUTINE COMPUTE	REAL KTI.KT2.KUP.1MP.LO COMMON RUNOFF(800).HYDR NAMF(500).RF(500).PEI(5 NAMF(500).RF(500).PEI(5 NAMF(500).RF(500).PEI(5 NAMF(500).RF(500).	> 114E(50).TF(00).T > 0UMX(4).DUMY(4).EGX.0 > 1PLOT(1).UMIX.NPAFINPAF > TITE(20). OMI.SHAPE.S < TLOAD.DLO.	S=(1000-0/CH)-10.0 00 120 1=1.NPRF 1F(1 - EQ. 1) GO TO 60 60 TO 100		SRO(I) = 0.0 PF(I) = SRO(I) PF(I) = PF(I) = 0.0 GO TO = 0.0	IF (RF(I) .6T* PHI) ARF IF (RF(I) .LE. PHI) ARF(I) .6T* 0.2*S) 6 SRO(I) = SRO(I-1) 60 TO 115	SRO(1)= (ARF (1)-0.2*S)* PE (1)= SRO (1-1) PE (1)= PE (1)-SRO (1-1) PE (T = VERY SMALL VALUE IF (PE (1) .61001) G PE (1)= 0.0	CONTINUE RETURN END
SUBROUTINE SCS	00 000	ပပ			60 65 70	25 80 80	105	C IIIO	120
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10/06/80										
FTN 4.8+518	OR THE PARAMETERS	.BASIN(\$00) BASIN(\$).BDATE: KT2.KUP.RN.ITOTAL.	116.TL.KT23 ), BSFLOW (500) pp. [SIM	IAT THE DATA IS INCOMPATABLE				;=UR*(T3—T2)) A jp#,8X,*UR*1,8X,*T1*1,8X	;•UR•(T3-T2))	//) 1734,6X,"AREA"/6F10.3/
	SUBROUTINE SIMLAT THIS SUBROUTINE IS A SIMULATION PROCEDURE FOR THE OF THE TVA DOUBLE TRIANGLE MODEL	IMP.LOSS.KT23.KUR ).HYDRO(800).RNO PEI(500).PE(500).RA .U.11.T2.T3.UP.UR.REI.KT1.	*UHYDRO (AGO) *NPRO CNOPT.NB BOO. TFLOW (SPOP AFT DW (STO BIO. TFLOW (SPOP AFT DW (STO BIO. TO SEE SEE STOR (STO BIO. SEE SEE SEE SEE SEE SEE SEE SEE SEE SE	OL PARAMETER TO SIGNIFY TH ING USED	8 0	7 = 11+0T	/UR Sonable value GO TO 310	110	(0,5*(UP+UR)*(T2-T1))+(0,5	TEO HYDROGRAPH PARAMETERS:: \$11,12-13,4REA \$"UR!"\$8x,"T!"\$8x,"T2",8x,"
4 0PT=1	SIMLAT UTINE IS DOUBLE	72.KUP.] OFF (800) RF (500)	AX.NPRT	AD CONTRO	+RN) UP.TI.T	T ( T2 . DT	TZ (UP)	1 (13,01 1 (2) (1 1 (2) (1 0) REI 1 (7X,18,8)	FA) +UP	# AD COS
14/74	SUBROUTINE SIMLAT THIS SUBROUTINE I OF THE TVA DOUBLE	AL KTISK MMON TISK RF (500)	RASH 1DM IME (500) UMX (4) 00 PLOT 1UN	RASH IS TH THE H RASH HO		** 17   1   1   1   1   1   1   1   1   1	# T1+(2 MIT T3 T 15=15.0# (T3 .LT	LL ADJUS (13 °LE (13 °LE EA=(0.5* ITE(6,36 RMAT(6X*	#(1.0/AR #(1.0/AR EA#(0.5# 17E(6.39	FORMAT(10X WRITE(6.40 FORMAT(6X. //) RETURN
SIMLAT	S FO	WO 44		FOHOS	5-55-56 5-55-56	-0-05 -0-05	⊢⊒⊢∺⊢ ⊢⊒⊩∺⊢	310 CA 3820 AAA 360 AAAA	224 F	395 FO 400 FO 430 PE
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1=1		SUBMOUTINE ADJUST(1.01) THIS SUBMOUTINE ADJUSTS TI.TZ.T3 TO THE NEAREST MULTIPLE OF DT		DELTA . 60 TO 20	DEL.	TA=DELTA/10.0	-(FDELTA/2.0)	
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10/06/80	ED HYDROGRAPH IF A	2	•	(RISING,FALLING,		(RISING,FALLING							
FTN 4.8+518	L CALCULATES THE ORDINATES OF A CONVOLUTED HYD AND A PRECIPITATION EXCESS MATRIX ARE PROVID	KT23,KUR,LFE,LMN,LCA,LMG,LSO,LAL 100),RNOFF (500),RAIN(500),STAGF (500 *PE(500),SRO(500),BASIN(4,BDATE, -105,105,105,000,STE,	\colony in the colony in	S ITS IN EACH LIMB OF HYDROGRAPH (RIS	ISE IN CONTROLLING OUTPUT	DS IN EACH LIMB OF HYDROGRAPH (RIS	1 TO 100	1 70 120	1 TO 140	ES AT EACH DT	c	c	c
74/74 OPT=1	ROUTINE CONVO	KT1.KT2.KUP.IMP.LOSS. MON RUNOFF (800).HYDRO(8 F1560).PEI (500).	ASH 10M A N N N N N N N N N N N N N N N N N N	OAD,ULOAU HON /CAPT/ICHAR,PF,PI,P PUTE NUMBER OF INCREMEN TREEESSION)	TA2=T2/DT TA3=T3/OT UT DUMMY PARAMETER TO U TD=1	BUTE NUMBER OF DT PERIO	N1+1 (N1 GT. DELTA1+0.1) GO (N1-1	N2-1 (N2-1 10 110 DELTAZ-0.1) GO N2-1	UN3+1 (N3 -67. DELTA3+0.1) GO 10 130 N3-1	COMPUTE HYDROGRAPH ORDINATES \$1=(UP/T] \$2=(UR-UP)/(T2-T]) \$3=(0,0-UR)/(T3-T2) HYORO(1)=0.0	N1+1 150 I=2+N1 RO(I)=HYDRO(I-1)+(S1+DT RIVUE N1+1	N2+1 160	N3-1 N0(I)=WYDR0(I-1)+(S3+DT
SUBROUTINE CONVOL	00 0000	AZZ Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z			28 20 20 20 20 20 20 20 20 20 20 20 20 20		000 I	110 NZ==	130 N34		150 CTON	I GANG	1 0 ×
SUBROUT		10	15	20	52	<b>e</b> 65	35	<b>•</b>	\$ <b>*</b>	20	55	09	

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	RAINFALL D. KILL JOB")														<u>.</u>		
	CESS 93 CEEDE						-JMIN) *PE(K)	FF (JJ)	000						368 )/(3600.*12	1 60 10 370	2
	06RAPH WIT	0(1)	×	×	O(I) *PE(1) F	N	AX DRO(JJ-JMI	AX 0(JJ)+RUNO	COOKER CONTRACTOR			-1)+DT	DMAX	• IDMAX	0) GO TO 66.*43560. (I) *CONVRT	(I) *CONVRI 6 61. PRPFAK	
CONTINUE HYDRO(N3) = 0.0 WRITE(6.185) FORMAT(///)	CONVOLUTE HYDROGRAPH WITH EX- ITOTAL=N3+NPRF-1 IF (ITOTAL - LT. 800) GO TO 1 MRITE(6-192) FORMAT(75x**STORAGE SPACE EX-	157=1 10 451 194 1=1.N3 0RO(I)=HYDR	TINUE 195 I=1.IDM RO(I)=0.0	196 I=1 • IDM 0FF (I) =0 • 0 TINUE	197 I=1•N3 90(I)=UHYDR TINUE 300 J=2•NPR	N=L-1 X=LMIN+N3 200 (L-1*LM OFF (LL)=0.0	210 JJ=K+JM QFF(JJ)=UHY	220 JJ=1.JM 220 JJ=1.JM 70(JJ)=HYDR	TINUE TINUE GAITOTAL	AX H O O O O O O O O O O O O O O O O O O	8=0.0 R=0.0 F(1)=0.0 RO(1)=0.0	340 1=2.NAI	#ITOTAL+1 350 [#]71.8 RO([]=0.0	INUE   O] = NPRO+1   360	INUE   IUNIT	FF (1) = RNOFF TINUE 410 I#1 • NB1	TO 380 EAK=HYDRO(I
7.0 85	~	6	* 025 000±0	9 4	197 2000 197 197	1 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		2 6	2000 2000 2000 2000 2000 2000 2000 200	900 C	PATE PHI		-	<b>.</b>		365 365 368 368 368 368	370 60

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	; (LBS) DISCHARGED FROM WAT [5.3]									0,550,CAL,LAL	"TE LOAD (LBS)":FISSS: "MG LOAD (LBS)":FISSS: "MG LOAD (LBS)":FISSS: "SO4 LOAD (LBS)": "SO4 LOAD (LBS)": "\12X,"ALK LOAD (LBS)":
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SUS=CONC*HYDRO(!)*DLOAD BEITE(6,4,4!) SUS FORMAT (1H+,4+X*F15,3) IF(IUNIT*E0.1)*SUS*SUS*3600. SUSS=SUS*SUS*SUS*SUS*SUS*SUS*TRUE CONTINUE CONTINUE VRO*SUSSY(CONC*DLOAD)	IF (ILOAD EG 0) RETURN MRITE (6460) SUSS FORMAT ('/) ISS 101 FENED ''/ IZX "DURING MRITE (6.460) GO TO	FORMAT (1H .//) GO TO 4109 CONTINUE CONTINUE IF (ICHAR.EG.1) GO TO	60 T0 780 ULFE=0.442+PT-0.568+U' CFE=4.42+ULFE LFE=ULFE=SR0(NPRF) +SQI	CENE 4 + 42 + 12   12   12   13   13   14   15   15   15   15   15   15   15	ULMG=0.0597*PI-0.113*PCMG=4.42*ULMG CMG=4.42*ULMG LMG=ULMG=SR0(NPRF)*S0 ULSO=0.719*PI-0.0216	LSO=1.450=SPC (NPRF) +SQI LSO=ULSO=SPC (NPRF) +SQI ULAL=0.319+P1=0.128+U- CAL=4.42+ULAL LAL=ULAL+SRO (NPRF) +SQI	60 TO 790 ULFE=0,323+PS+0,12 CFE=4,42+ULFE CFE=ULFE=580(NPF) +SQU ULMN=0,0167+PS+0,002	CAN #4 .42*ULMN LMN=ULMN=SRO (NPRF) *SQU ULC #10.131*P5+0.30 CCA #4.42*ULCA LCA #ULCA #SRO (NPRF) *SQU	ULSG=10, 4247540.40 CMG=46,424CLMG CMG=46,424CLMG CLSO=10,394PS+2.45 CSO+4,424CLMG CSO+4,424CLMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO+4,424CMMG CSO	L30=0L30=3x0 (NFT) = 38 CAL = 4 = 40 = 48 LAL = 0LAL = 5R0 (NPR) = 50 WITE (6.791) CFE = LFE = 50	
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ASSUME 74/74 OPT=1	HAT. MIST. TWENCE AND THE PROPERTY.	TETTSTEE TO SO TO 10	FORMAT (46%)	*36x***********************************	60 TO 12 60 TO 12 60 WRITE (6-15) 5 FORWAT (46Kx	* HRY SETTING ISIM = 1 YOU HA >36x* HTO SUPPLY ACTUAL OR DE >36x* HESTIMATE OF THE SCS CO	IZ CONTINUE IF(IUNIT-EG-1) GO TO 25	20 FORMATIGEX, "OUTPUT UNITS OF 36X, MAND INCHES AS SELECTED	5 #FITE(6.30) 30 FORMAT(46x,"OUTPUT UNITS OF >36x,"AND CUBIC FEET PER SE(	>36X,"!UNIT = 1.",/) 32	WRITE(6.35) 35 FORMAT(46X."NO PLOT HAS BEE	40 WRITE(6.45) 45 FORMAT(46x,"YOU HAVE REQUESTED TH >36x,"BY SETTING IPLOT = 1.",/)	17 CONTINUE IF (ICHAR.61.0) GO TO 55	SO FORMAT (46X,"THE WATERSHED I >36X,"STORM SEWERS; ICHAR = 60 TO 100	55 IF (ICHAR-61-1) 60 TO 65 WRITE (6-60)	> FUNDATION AND TENDED	VO FORMAT (46X,"THE WATERSHED >36X,"MAREA, OR DENUDED FORE	60 TO 100 75 IF(ICHAR.6T.3) GO TO 85 WRITE(6.80)	30 FORMAT(46X,"THE WATERSHED 3	33 MRITE(6-540) 30 FORMAT(46X*"THE WATERSHED   3-36X***FOU HAVE SET ICHAR = 3-36X***********************************	IF (ILAG.61.0) GO TO 110 WPITE(6.105) 5 FORMAT (46x."YOU HAVE DECIDE	×36X
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110 IF(I LAG-67.1) GO TO 120

115 FORMAT (46.1.5) "YOU HAVE ELECTED TO SIMULATE LAG MODULUS IN"."

> 36.x" MAND PERCENT IMPERVIOUS AREA OF THE WATERSHED AREA"."

> 36.x" MAND PERCENT IMPERVIOUS AREA OF THE WATERSHED AREA"."

120 MRITE (4.12.5)

125 FORMAT (46.X" "YOU MAVE ELECTED TO SIMULATE LAG MODULUS IN"."

> 36.x" MADONES ASSUMING IT IS A FUNCTION OF THE ROBAL WATERSHED"."

> 36.X" MADONES ASSUMING THE BERGENT OF THE WATERSHED WATERSHED"."

130 TO THE WATERSHED "."

130 TO THE WATERSHED "."

131 FORMAT (46.X" "SEDIMENT LOAD HAS BEEN SIMULATED ASSUMING THE"."

> 36.X" MATERSHED IS RURAL OR URBAN WITHOUT WAJOR CONSTRUCTION:"."

> 36.X" MATERSHED IS RUBD HAS BEEN SIMULATED ASSUMING THE"."

> 36.X" MATERSHED IS RUBD HAS BEEN SIMULATED ASSUMING THE"."

> 36.X" MATERSHED IS A DENUBDED FOREST AREA OR A COAL STRIP WINE".

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261	FSOP	.63x."INTRODUCTION".//.35x, 1R FORCE RUNDEF MODEL (AFRUME SIMULATING STORM"./.	_	AND THE RESEARCH AND DEVELOPMENT DIRECTORATE OF THE", WGINEERING SERVICES CENTER (AFESC), TYNDALL AIR FORCE"	DA. DR. DONALD E. OVERTON OF THE UNIVERSITY OF TENNES #45 THE PRINCIPAL INVESTIGATOW AND DR. GEORGE M. SCH 35X,"0F THE U.S. AIR FORCE WAS THE PROJECT OFFICER.	"AX","SEPARATE BUT COMPLIMENTARY STUDIES WERE INCORPORAT "A"," 35X" 14REE STANDIES HAD THE FUNDAMENTAL OBJECTIVE TO EVALUA THE EFFECTS OF SPECIALIZED LAND USE ON STORMWATER RUNO **35X**	OUÂLITY. THE THREE STUDIES WERE FUNDED BY THE U.S. DEP RGY. THE U.S. DEPARTMENT OF INTERIOR AND THE U.S. AIK XX. REDIOD WAS FROM 1975, THROUGH 1979, AFRIN WAS DEVEL-U.	COURSE OF ANALYZING 410 STORM EVENTS ON 36 WATERSHEDS UDED AGRICULTURAL, URBAN AND FORESTED LAND USE CONDITI	COAL STRIP MINING AND THREE WATERSHEDS ON A U.S. AIR F "BASE.".//) 34x,	FYELOPED FOR THE PURPOSE OF SIMULATING STORMWATER",/, APHS FROM ACTUAL OR DESIGN STORM RAINFALL DISTHIBUTION ARACTERISTICS AND SOIL TYPE IN THE WATERSHED OF CONCER	*3 "HE REQUIRED INPUT BASIN CHARACTERISTICS ARE:",', 355x"   PERCENT OF WATERSHED THAT IS FORESTED",' 555x"   2 PERCENT OF WATERSHED THAT IS IMPERVIOUS",' 35x"   3 PERCENT OF WATERSHED THAT IS DENUDED OR STRIP MINEO" 50 **,35x"   4 SOURFEC DAALNAEL EXCESS TIME DISTRIBUTIO	L L		SERVA SERVA SINU HICH	"ICESS HYETOGRAPH. AFRUM ALSO SIMULATES POLLUTANT LOADS FOR THE"/, "HINPUTED STORM. THIS SIMULATION IS A FUNCTION OF THE SPECIFIED",/, 235x,"WATERSHED AND STORM CHARACTERISTICS.",/,1H]) RETURN
76/74	SUBROUTINE AE	FORMAT(15(7)		. "KNOXVILLE # /35X, /35X, /35X, /35X,		FORMAT (115.30)	**ASSOCIATED 0 *ARTIN*/*35X* **MENT OF EWEN *FORCE **/*35X* **THE OF THE		**************************************	. "AFRUM WAS DE .35X,"HYDROGRA .S."./.35X, ."LAND USE CHA		MARE SIMULATE ON:) WRITE (6.4000 WRITE (11.93	## 5001ATED ## 14 / 4 35 X + 14 / 4 35 X + 15 / 4 7 X + 15 / 4		**************************************
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